

**TAMARACK NEIGHBORHOOD DRAINAGE  
PRELIMINARY DESIGN HYDRAULIC REPORT**

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**Prepared for:**

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**April 2013**

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## Introduction

The City of Sammamish is interested in upgrading stormwater facilities in the Tamarack neighborhood to resolve existing drainage problems and support future development. The Tamarack neighborhood has localized drainage problems in the vicinity of 209<sup>th</sup> Avenue NE and erosion in open ditches along NE 4<sup>th</sup> Street. Planned development in this neighborhood is not supported by the existing privately installed infrastructure. The goal of the proposed drainage improvements is to provide solutions to existing drainage problems and support future development without causing impacts to natural resources or existing infrastructure.

This report documents the Tamarack Neighborhood Drainage Preliminary design. It is a follow up to Design Alternatives for Inglewood and Tamarack Neighborhood Drainage Projects, Technical Memorandum by Windward Environmental, December 2011 (Windward Memo). Osborn Consulting, Inc. (OCI) is a subconsultant to Windward on this project. The Windward Memo documents project background, existing conditions, and qualitative alternatives analysis. This report provides a brief summary of the project background and existing conditions, hydrologic and hydraulic analysis, and documentation of the preliminary design. The Tamarack Neighborhood Drainage Preliminary Design includes:

- Proposed collection and conveyance system that improves the existing drainage issues and allows for future development of property.
- Discharge to Lake Sammamish via existing ditch-culvert system on Louis Thompson Road. King County flow control exemption shall be verified during final design.
- Consists of 1,265 linear feet of new pipe, 1,738 linear feet of upsized pipe, and 14 new catch basins.
- Provides enhanced water quality treatment for 0.5 acres of City right of way.
- Estimated project cost is \$974,000.

## Existing Condition summary

Evaluation of the current site conditions consisted of a review of existing utility plans and maps, review of GIS data, field reconnaissance, and survey data. The existing site can be divided into four areas: the Tlingit Subdivision Section, the Private Road Section, the 209<sup>th</sup> Avenue NE Section, and the NE 4<sup>th</sup> Street Section. Details for the sections are outlined in **Table 1**. Plan sheets are provided in **Attachment A**.

<b>Table 1: Section Details</b>				
	<b>Tlingit Subdivision</b>	<b>Private Road</b>	<b>209<sup>th</sup> Ave NE</b>	<b>NE 4<sup>th</sup> Street</b>
<b>Vicinity</b>	205 <sup>th</sup> Ave NE, 206 <sup>th</sup> Ave NE, and NE 5 <sup>th</sup> Place	Grassy area and steps between NE 5 <sup>th</sup> Place and NE 4 <sup>th</sup> Street	209 <sup>th</sup> Ave NE from NE 4 <sup>th</sup> Street to cul-de-sac	NE 4 <sup>th</sup> Street from 211 <sup>th</sup> Ave NE to Private Road
<b>Existing Storm Drainage</b>	1,367 LF 12-inch pipe, 171 LF 72-inch pipe, 134 LF 18-inch pipe; 4 MHs, 8 CBs	299 LF 8-inch pipe; 2 MHs, 3 CBs	386 LF 12-inch pipe; 1 CB	91 LF 12-inch pipe/culvert; 2 CBs; ditch flow
<b>Information Resources</b>	Previous utility replacement/ extension plans, survey data	Survey data	GIS data and field reconnaissance	GIS data and field reconnaissance
<b>Plan Sheets</b>	1-3	4	5	6

The ditch along the north side of NE 4<sup>th</sup> Street is armored with large rocks, but shows signs of erosion and degradation due the amount of runoff.

## Design Summary

The preliminary design is based on Alternative B in the Windward Memo (See **Figure 1**). The preferred alternative from the memo was Alternative C, where the proposed storm pipes on NE 4<sup>th</sup> Street tightline through a private ravine at the west end. Alternative C, however, is not possible as the owner of the property with the ravine is building a house on the property. The preliminary design based on Alternative B connects to the existing stormwater conveyance system in the Tlingit subdivision beginning on NE 5<sup>th</sup> Place. The Preliminary Design is comprised of a proposed collection system and an upsizing of the existing collection system. These design elements, typical design assumptions, and the cost estimate are described below. The Tamarack Neighborhood Drainage Preliminary Plans are included as **Attachment A**.

### New Collection System

A new collection system is proposed in the Private Road, 209<sup>th</sup> Avenue NE, and NE 4<sup>th</sup> Street Sections. **Table 2** describes the proposed components.

<b>Table 2: New Collection Systems</b>			
	<b>Private Road</b>	<b>209<sup>th</sup> Ave NE</b>	<b>NE 4<sup>th</sup> Street</b>
<b>New Conveyance</b>	346 LF 18-inch pipe	10 LF 12-inch pipe, 200 LF ditch	909 LF 12-inch pipe
<b>New Structures</b>	1 CB Type 2 48-inch	1 CB Type 1	1 CB Type 2 48-inch, 9 CB Type 1
<b>New Water Quality</b>	0	0	5 Filterra® Structures
<b>New Curb &amp; Gutter</b>	0	0	1,590 LF

Additional information about the proposed NE 4<sup>th</sup> Street collection system is bulleted below.

- The existing rock-lined ditch along the north side of NE 4<sup>th</sup> Street will remain in place.
- The new collection system along NE 4<sup>th</sup> Street will divert stormwater from the ditch to the pipe system and alleviate the existing erosion in the ditch.
- Filterra® units are proposed offset from the catch basins behind a concrete flume on alternating sides of the street.
- New curb and gutter are included in the construction cost estimate and on the plans. New curb is essential to the performance of the proposed water quality treatment system.
- Clean water from private residential development can connect at tap into the 12-inch collection line directly, or stub outs from the overflow catch basins to the property line can be added during final design. Design of future development connections will be the responsibility of private property owners.
- Two new 18-inch stormwater pipes and one Manhole Type 2-48" connect the proposed system along NE 4<sup>th</sup> Street (via the Private Road section) to the existing system on NE 5<sup>th</sup> Place (in the Tlingit Subdivision section).

### Upsize the Existing Collection System

The existing collection systems in the Tlingit Subdivision and Private Road Sections are currently undersized to manage the stormwater from the new collection system. The proposed changes are in **Table 3**.

<b>Table 3: Upsized Collection Systems</b>		
	<b>Tlingit Subdivision</b>	<b>Private Road</b>
<b>Upsized Conveyance</b>	1,402 LF 18-inch pipe, 134 LF 24-inch pipe	137 LF 12-inch pipe, 65 LF 18-inch pipe
<b>Replaced Structures</b>	1 CB Type 2 48-inch	1 CB Type 1

### Typical Design Assumptions

The Preliminary Design was developed using survey, as-built, GIS and field observation data. The proposed alignment in the NE 4<sup>th</sup> Street Section flows on the south curb of NE 4<sup>th</sup> Street until it meets up with the Private Road Section. Sewer lines run approximately down the middle of the road in the NE 4<sup>th</sup> Street Section. Water lines are also present along NE 4<sup>th</sup> Street. Sewer and stormwater pipes cross once at Private Road Section, and several times within the Tlingit Subdivision Section. No information exists about other utilities in the Tlingit Subdivision Section. Utility conflicts shall be determined and addressed during final design.

The proposed stormwater system has steep pipe slopes. Pipe slopes range from 1.0% to over 25% but are typically in the 10% to 20% range. Proposed pipe slopes generally follow the slope of the topography and some pipe slopes were modified to improve conveyance capacity.

The proposed pipe type is PVC, however, any WSDOT schedule A smooth interior wall pipe is acceptable. Conveyance systems were analyzed using a Manning's roughness of  $n=0.013$ , so additional hydraulic analysis would be required prior to approval of corrugated pipe.

The minimum distance from structure rim to pipe invert is 4-feet. This distance ensures minimum 2-feet of cover over 18-inch pipe and compatibility with proposed water quality treatment facilities which have a rim to invert distance of 3.5-feet. Shoring or extra excavation is required for all depths of 4-feet or more. Several pipes have rim to invert depths greater than 4-feet to maintain minimum slopes for conveyance and/or maintain minimum cover over the pipe at sag locations.

### Project Cost Estimate

The preliminary design project cost estimate is \$974,000. The project cost estimate includes construction cost plus design cost.

Construction Cost:	\$740,000
Design Cost:	\$234,000
Total Project Cost:	\$974,000

The construction cost was established using unit prices for key design elements of the proposed design (i.e. pipe, structures, and site restoration) and includes a contingency of 30% and sales tax. Design costs including estimates for easement acquisition, engineering design, permitting, and construction management are included to estimate a total project cost. A copy of the cost estimate is included in **Attachment B**.

### ***Hydrologic and Hydraulic Analysis***

A hydrologic and hydraulic analysis was performed using Manning's Equation and flows for the 100-year storm event. Flows were determined by others using MGSFlood, and can be found in **Attachment C**. Existing conditions were analyzed to determine if the capacity of downstream pipes were sufficient for the current cumulative flow. Proposed conditions were determined to utilize the minimum slopes and pipe sizes to carry the cumulative flow for all sections. The proposed pipes were assumed to have smooth interiors and a Manning's coefficient (n) of 0.013.

## **Design Criteria and Results**

This section demonstrates compliance with the King County Surface Water Design Manual (SWDM), 2009, minimum drainage requirements as supplemented by Chapter PWS.20 Storm Drainage of the City of Sammamish Interim Public Works Standards (Ordinance No. O2000-60). King County Core Requirements 1 through 9 are outlined below.

### **1. Discharge at the natural location**

The natural discharge location is Lake Sammamish via an existing conveyance system along Louis Thompson Road NE. The proposed system connects to the existing conveyance system at 205<sup>th</sup> Avenue NE and Louis Thompson Road NE.

### **2. Offsite analysis**

Downstream pipes are assumed to be sufficiently sized to handle the additional flow. A downstream analysis shall be performed during final design.

### **3. Flow control**

Flow control is not required for project directly discharging to Lake Sammamish per King County SWDM Direct Discharge Exemption. A downstream analysis shall be performed during final design to confirm conveyance capacity from the project outfall to the lake.

### **4. Conveyance System**

Manning's Equation analysis of the Preliminary Design demonstrates conveyance capacity for the 100-year flow. King County conveyance requirements allow pipe system structures to overtop for runoff events that exceed the 25-year design capacity, provided the overflow from a 100-year event does not create or aggravate a severe flooding or erosion problem. This allowance may present opportunities to reduced pipe diameter and project cost during final design.

### **5. Erosion and sediment control**

Erosion and sediment control plans shall be completed during final design.

**6. Maintenance and operations**

Maintenance and operations plans shall be completed during final design.

**7. Financial guarantees and liability**

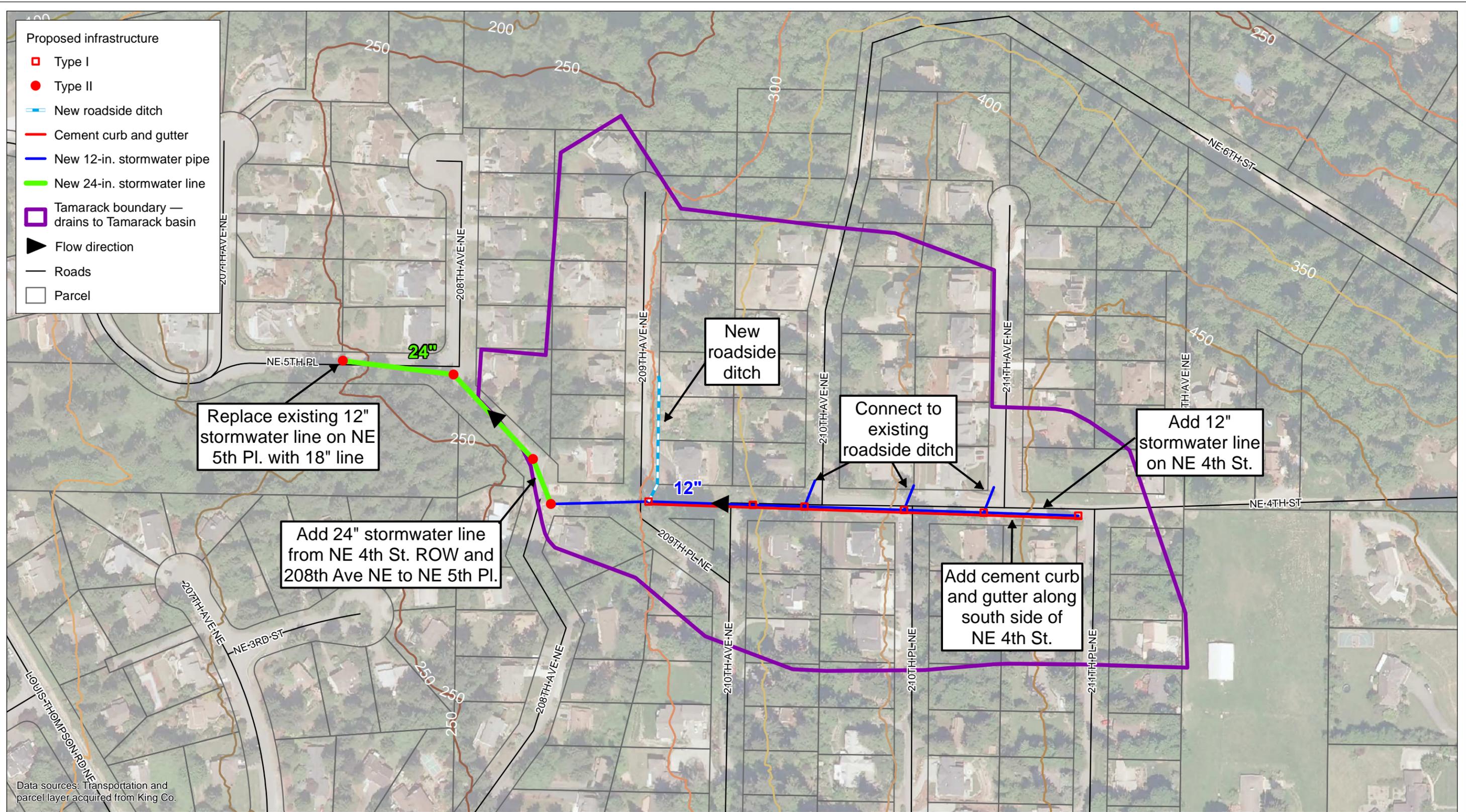
Not Applicable.

**8. Water quality**

Filtterra® Bioretention Systems were designed in accordance with the Engineering Design Assistance Kit (DAKit) v01-WA (select sheets included as **Attachment D**). A total of 5 Filtterra® units (4-ft by 4-ft) provide enhanced water quality treatment for City of Sammamish right of way (0.5 acres of pollution generating impervious surface).

Enhanced treatment is provided because Lake Sammamish is a King County Sensitive Lake Water Quality treatment area.

The size and quantity of Filtterra® units, or approved equal, shall be adjusted during final design. The Preliminary Design assumes water quality treatment for driveways will be the responsibility of private property owners.



**ATTACHMENT A**

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**TAMARACK NEIGHBORHOOD DRAINAGE  
PRELIMINARY PLANS**

# CITY OF SAMMAMISH

## TAMARACK NEIGHBORHOOD DRAINAGE PROJECT NO. XXXX PRELIMINARY PLANS



VICINITY MAP  
1" = 250'

**DRAWING LIST:**

- 0 - COVER
- 1 - 205TH AVE NE
- 2 - 206TH AVE NE
- 3 - NE 5TH PL
- 4 - PRIVATE ROAD & NE 4TH ST
- 5 - 209TH AVE NE
- 6 - NE 4TH ST
- 7 - DETAILS

**OWNER:**  
CITY OF SAMMAMISH  
ERIC LAFRANCE, PE  
SENIOR STORMWATER ENGINEER  
801 228TH AVENUE SE  
SAMMAMISH, WA 98075  
(425) 295-0562

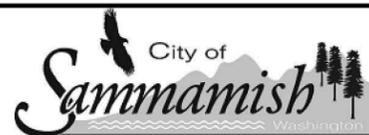
**SURVEYOR:**  
AXIS SURVEYING & MAPPING  
13005 NE 126TH PL  
KIRKLAND, WA 98034  
(425) 823-5700  
1-800-933-AXIS

**BACKGROUND SOURCE:**  
BACKGROUND INFORMATION SHOWN ON  
THESE PLANS ARE FROM:  
- CITY OF SAMMAMISH AND KING COUNTY  
PROVIDED GIS DATA  
- TOPOGRAPHIC SURVEY FOR SAMMAMISH  
TAMARACK BASIN PROVIDED BY AXIS  
8/29/12  
- AS BUILT WATER AND SEWER PLANS BY  
SAMMAMISH PLATEAU WATER AND  
SEWER DISTRICT 12/11/96  
- PLAT OF TLINGIT ADDITION PLANS BY  
MYRON ANDERSON & ASSOCIATES 7/83

SCALE BARS INDICATE SCALE OF  
FULL-SIZE (22 X 34 INCH) DRAWINGS.  
FOR REDUCED SIZE DRAWINGS  
ADJUST SCALE ACCORDINGLY.

PRELIMINARY PLANS – NOT FOR CONSTRUCTION

FILE NAME: C:\Projects\10-110024\_Sammamish\_SW\CAD\Tamarack\Tamarack.dwg  
USER: Marie  
PLOTTING DATE: 4/22/2013



**ENGINEER:** Osborn Consulting Inc.  
1800 - 112th Avenue NE, Suite 220E  
Bellevue, Washington 98004  
TEL: (425) 451 - 4009  
FAX: (425) 451 - 4901

DESIGNED BY: RDP  
DRAWN BY: MLP  
CHECKED BY: LCR  
DATE: DEC 2012

**TAMARACK  
STORMWATER IMPROVEMENTS  
COVER**

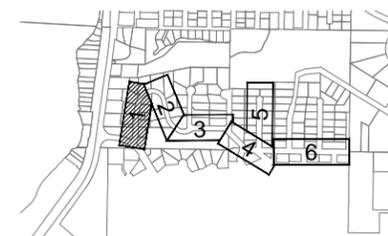
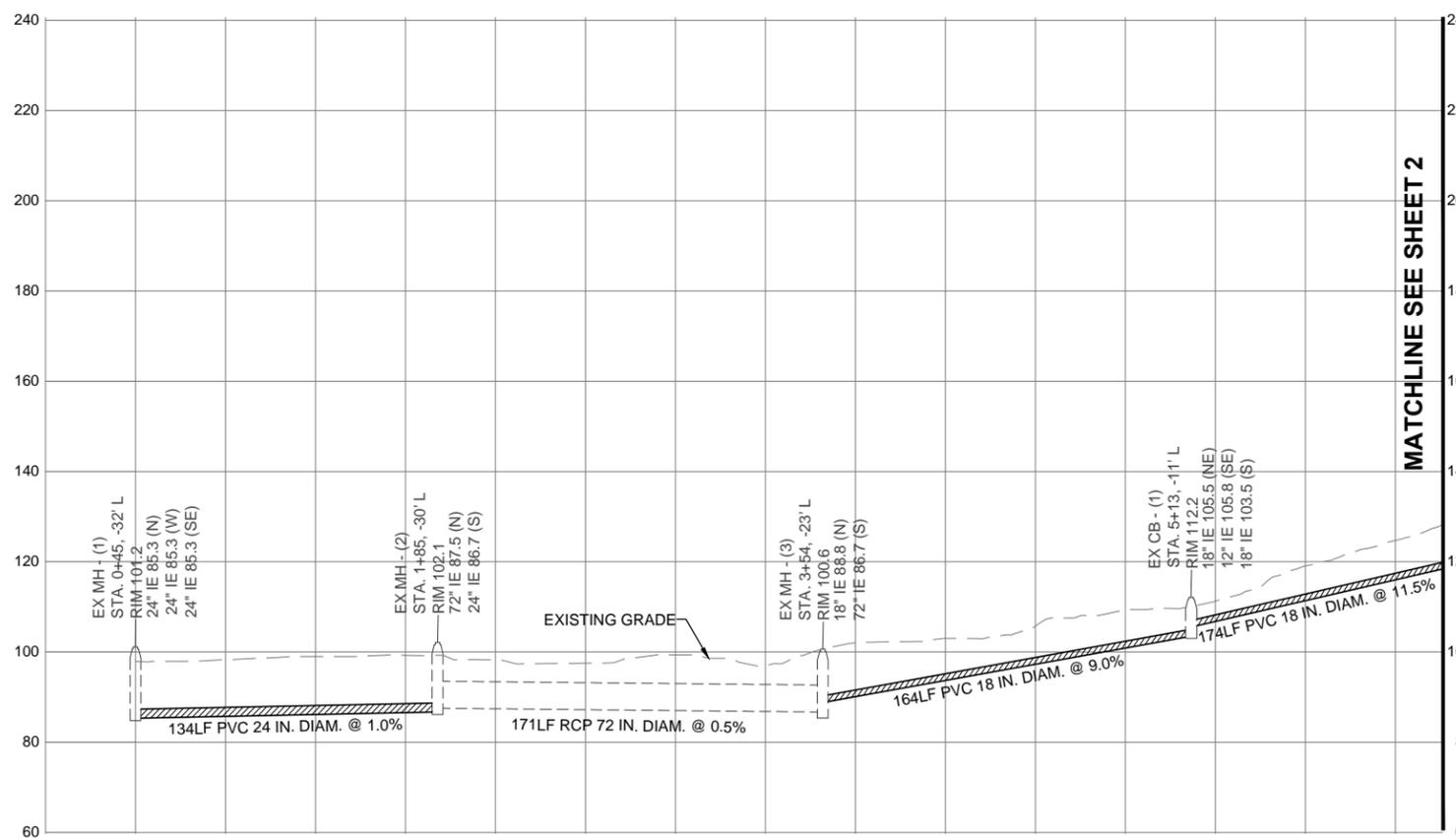
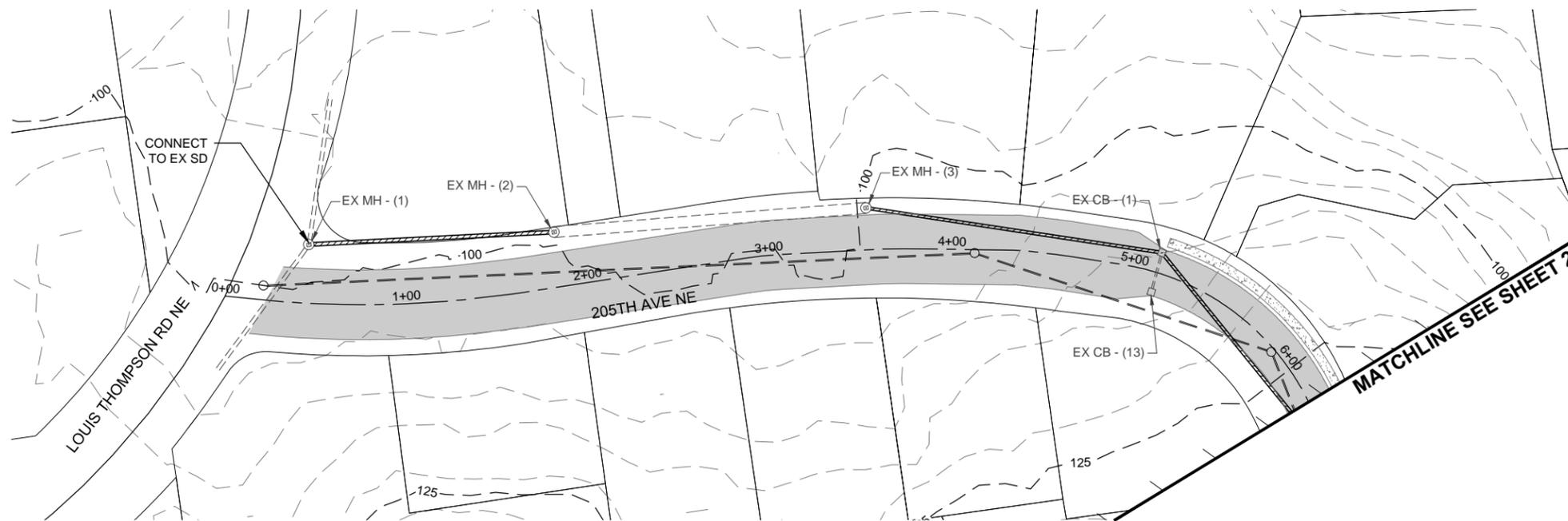
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OCI PROJECT NO.

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SHEET OF

NO.	DATE	BY	CKD.	REVISION

**LEGEND:**

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- 5 --- 5-FT CONTOUR (GIS)
- RIGHT-OF-WAY (GIS)
- ROAD CENTERLINE
- EXISTING PAVEMENT
- EXISTING SIDEWALK
- EXISTING SEWER MH
- EXISTING SEWER PIPE
- ⊙ EXISTING STORM MH
- EXISTING STORM CB
- EXISTING STORM DRAIN PIPE
- ⊙ PROPOSED MH TYPE 2
- PROPOSED CB TYPE 1
- PROPOSED WATER QUALITY TREATMENT 4-FT X 4-FT (SEE DETAILS SHEET 7)
- PROPOSED CONCRETE FLUME (SEE DETAILS SHEET 7)
- PROPOSED STORM DRAIN PIPE
- PROPOSED DITCH
- PROPOSED CURB AND GUTTER



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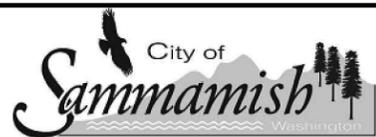
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 Bellevue, Washington 98004  
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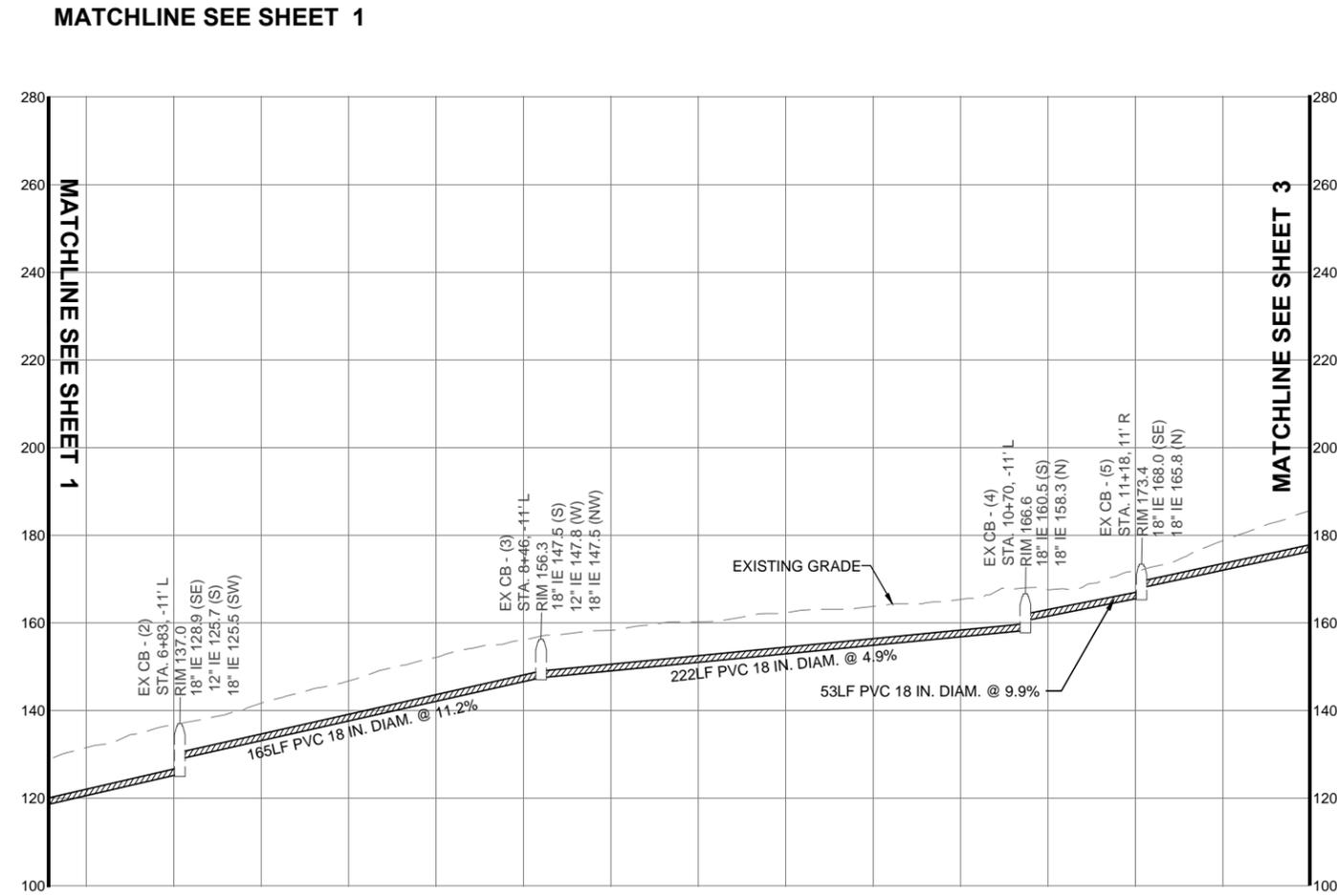
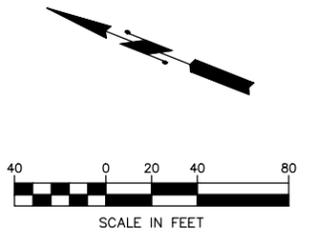
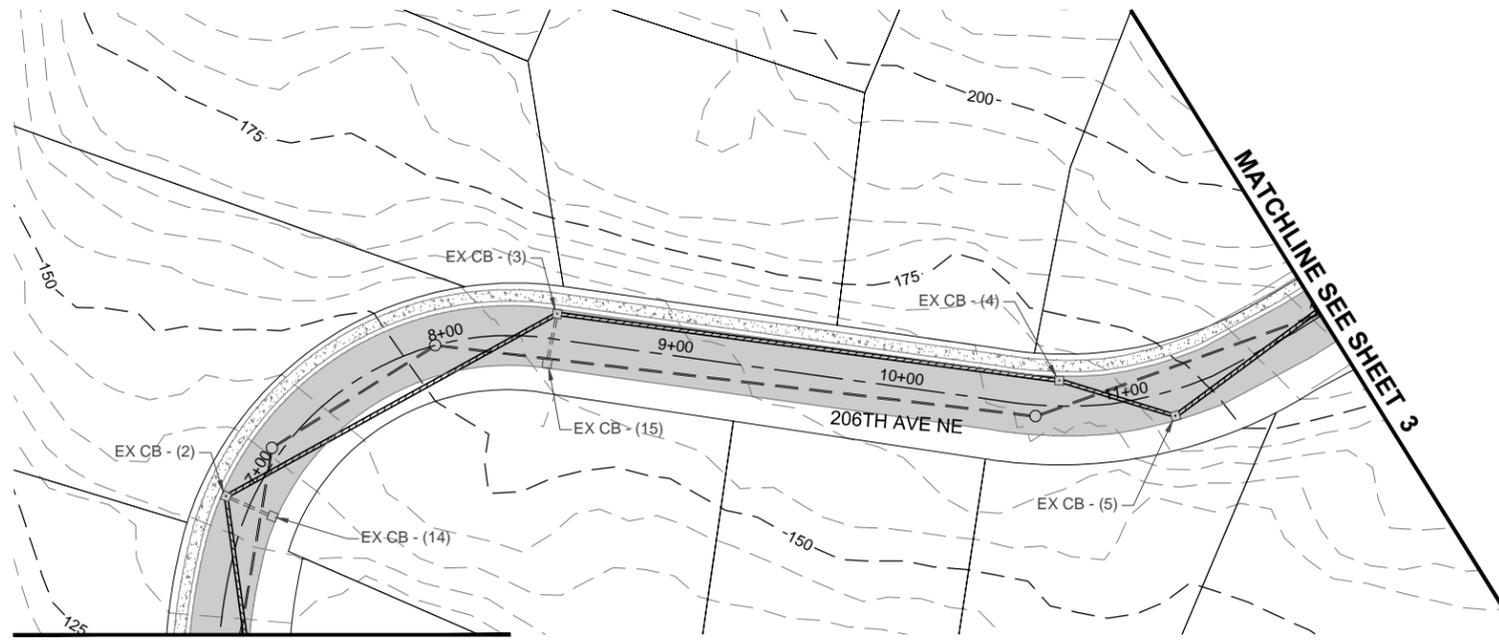
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**TAMARACK**  
**STORMWATER IMPROVEMENTS**  
**205TH AVE NE**

**10-110024**  
 OCI PROJECT NO.  
 1 SHEET OF 7

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-  5-FT CONTOUR (GIS)
-  RIGHT-OF-WAY (GIS)
-  ROAD CENTERLINE
-  EXISTING PAVEMENT
-  EXISTING SIDEWALK
-  EXISTING SEWER MH
-  EXISTING SEWER PIPE
-  EXISTING STORM MH
-  EXISTING STORM CB
-  EXISTING STORM DRAIN PIPE
-  PROPOSED MH TYPE 2
-  PROPOSED CB TYPE 1
-  PROPOSED WATER QUALITY TREATMENT  
4-FT X 4-FT (SEE DETAILS SHEET 7)
-  PROPOSED CONCRETE FLUME  
(SEE DETAILS SHEET 7)
-  PROPOSED STORM DRAIN PIPE
-  PROPOSED DITCH
-  PROPOSED CURB AND GUTTER

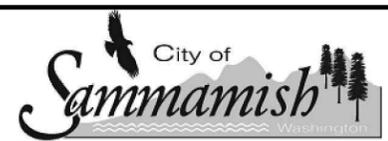


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**TAMARACK**  
**STORMWATER IMPROVEMENTS**  
**206TH AVE NE**

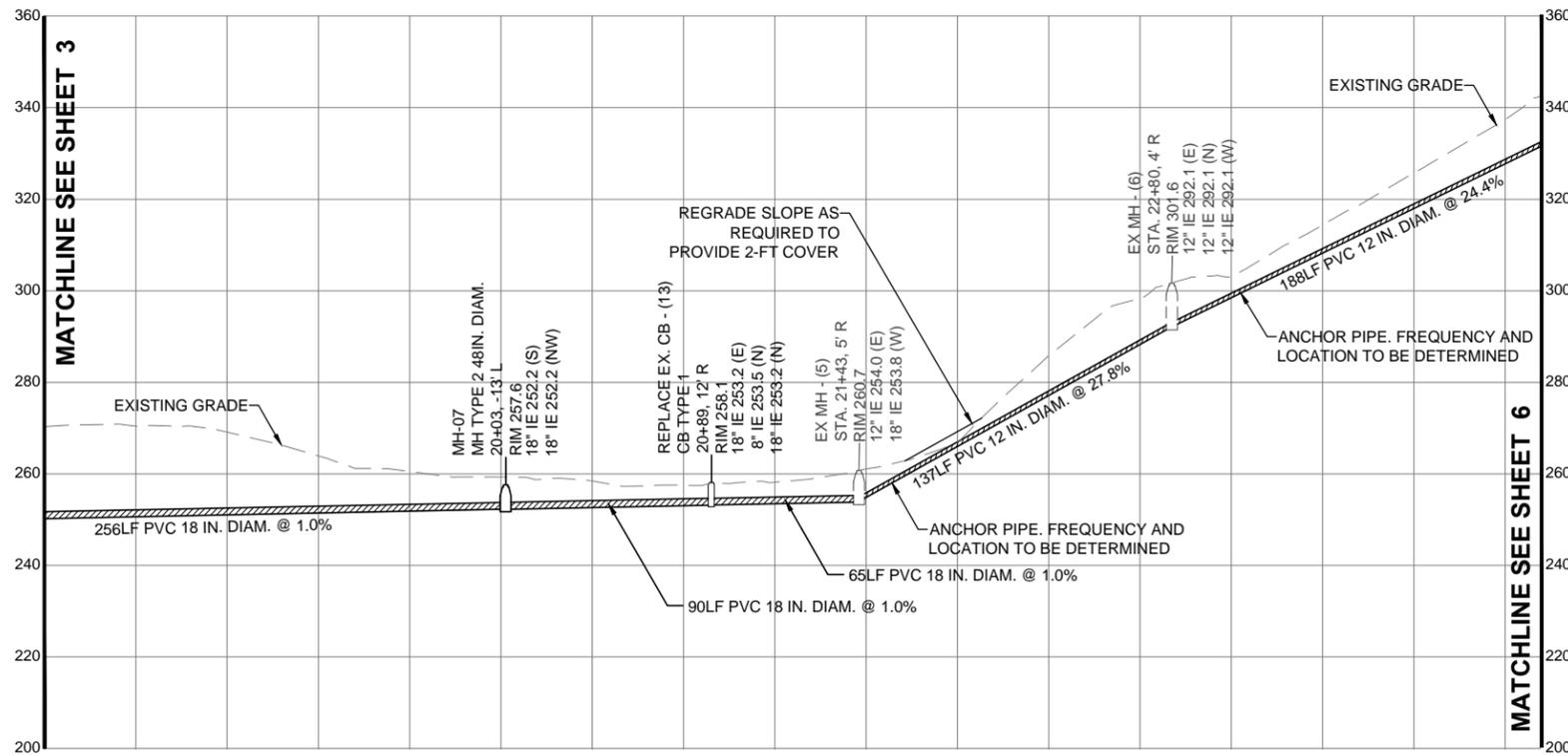
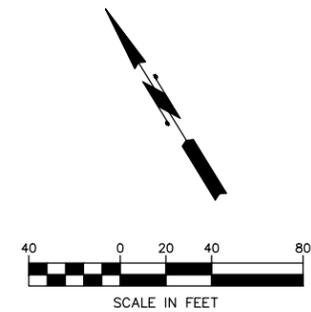
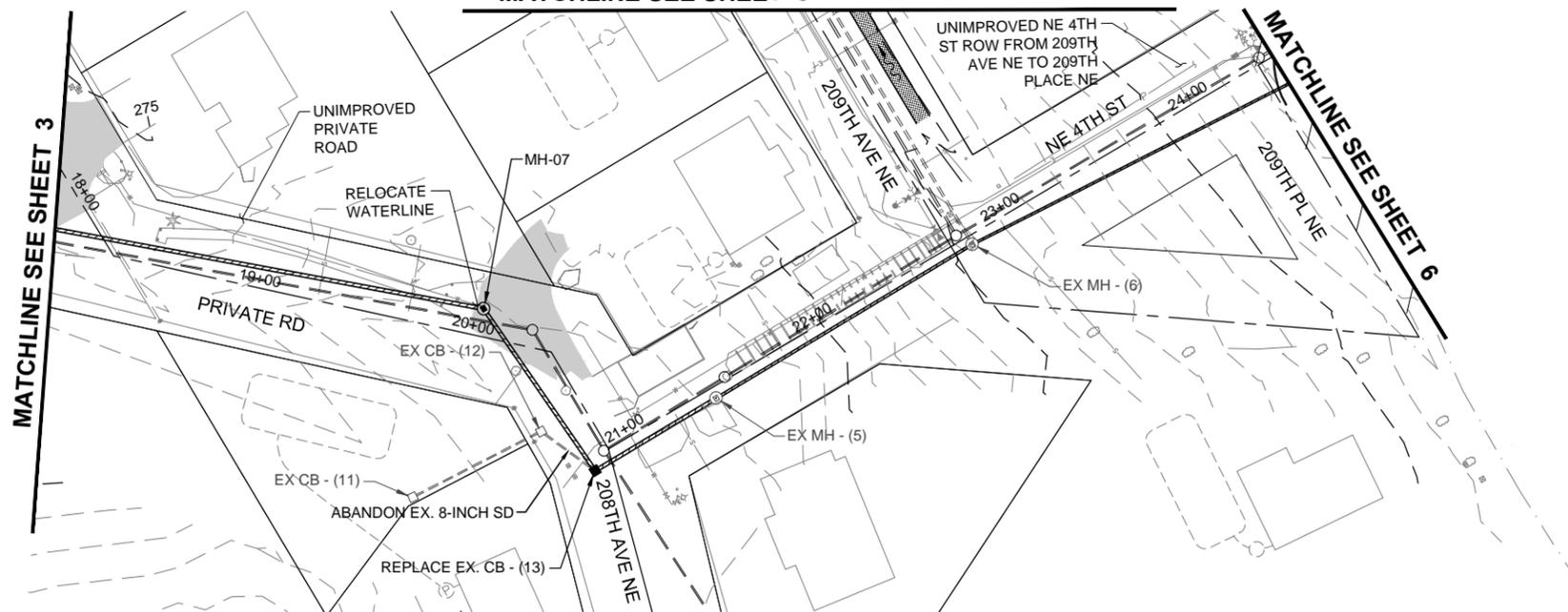
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**2** / **7**  
SHEET OF

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MATCHLINE SEE SHEET 5

- LEGEND:**
- 25--- 25-FT CONTOUR (GIS)
  - 5--- 5-FT CONTOUR (GIS)
  - RIGHT-OF-WAY (GIS)
  - ROAD CENTERLINE
  - EXISTING PAVEMENT
  - EXISTING SIDEWALK
  - EXISTING SEWER MH
  - EXISTING SEWER PIPE
  - ⊙ EXISTING STORM MH
  - EXISTING STORM CB
  - EXISTING STORM DRAIN PIPE
  - ⊙ PROPOSED MH TYPE 2
  - PROPOSED CB TYPE 1
  - PROPOSED WATER QUALITY TREATMENT 4-FT X 4-FT (SEE DETAILS SHEET 7)
  - PROPOSED CONCRETE FLUME (SEE DETAILS SHEET 7)
  - PROPOSED STORM DRAIN PIPE
  - PROPOSED DITCH
  - PROPOSED CURB AND GUTTER



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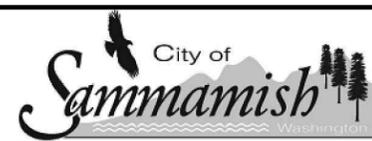
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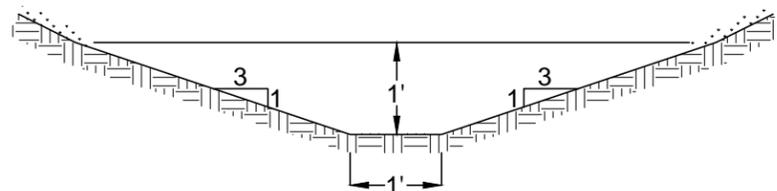
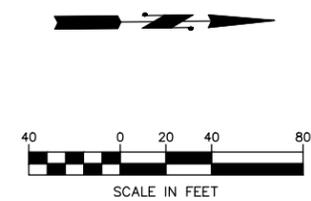
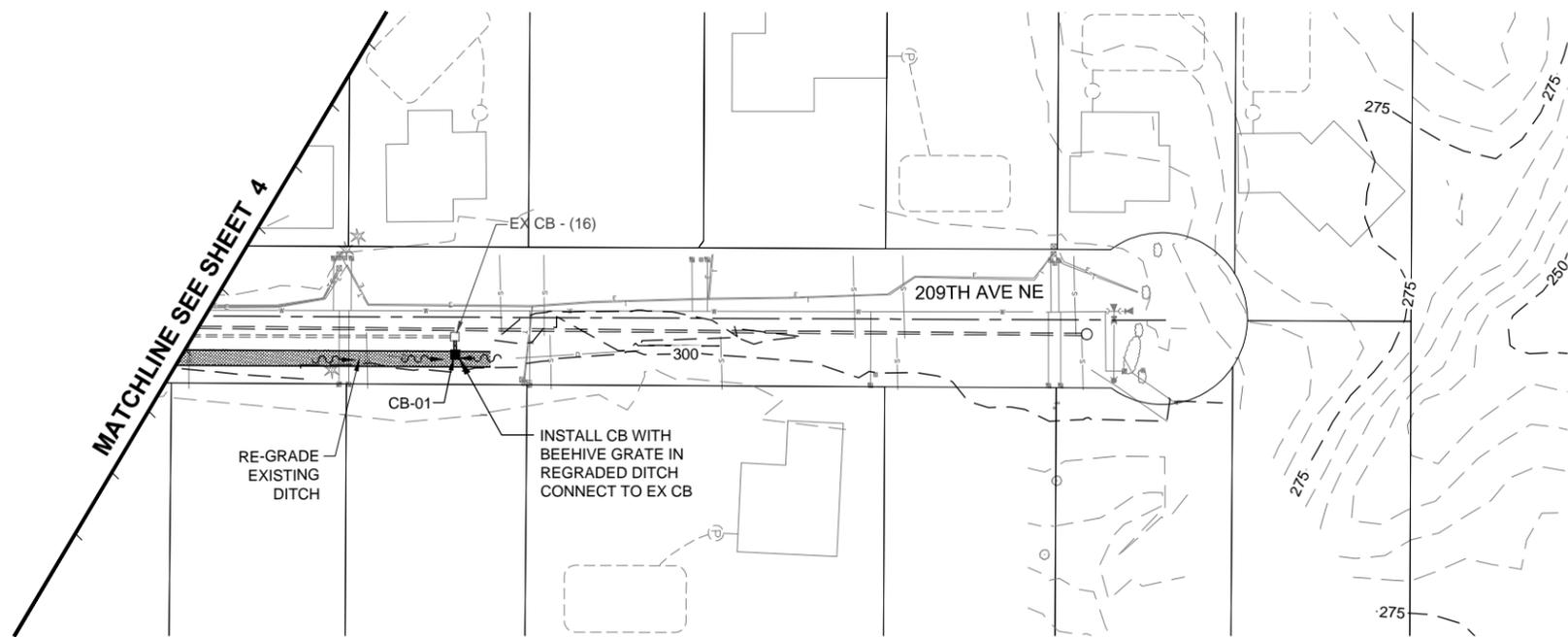
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**TAMARACK STORMWATER IMPROVEMENTS PRIVATE RD & NE 4TH ST**

**10-110024**  
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 4 SHEET OF 7

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-  5-FT CONTOUR (GIS)
-  RIGHT-OF-WAY (GIS)
-  ROAD CENTERLINE
-  EXISTING PAVEMENT
-  EXISTING SIDEWALK
-  EXISTING SEWER MH
-  EXISTING SEWER PIPE
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-  EXISTING STORM CB
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-  PROPOSED CB TYPE 1
-  PROPOSED WATER QUALITY TREATMENT  
4-FT X 4-FT (SEE DETAILS SHEET 7)
-  PROPOSED CONCRETE FLUME  
(SEE DETAILS SHEET 7)
-  PROPOSED STORM DRAIN PIPE
-  PROPOSED DITCH
-  PROPOSED CURB AND GUTTER



TYPICAL DITCH SECTION  
N.T.S



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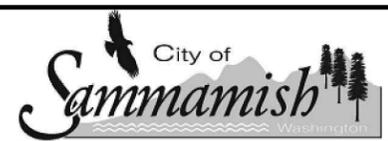
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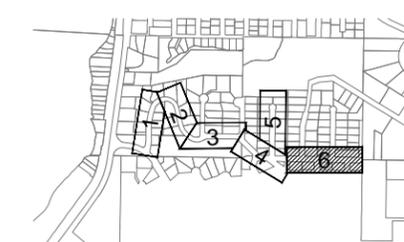
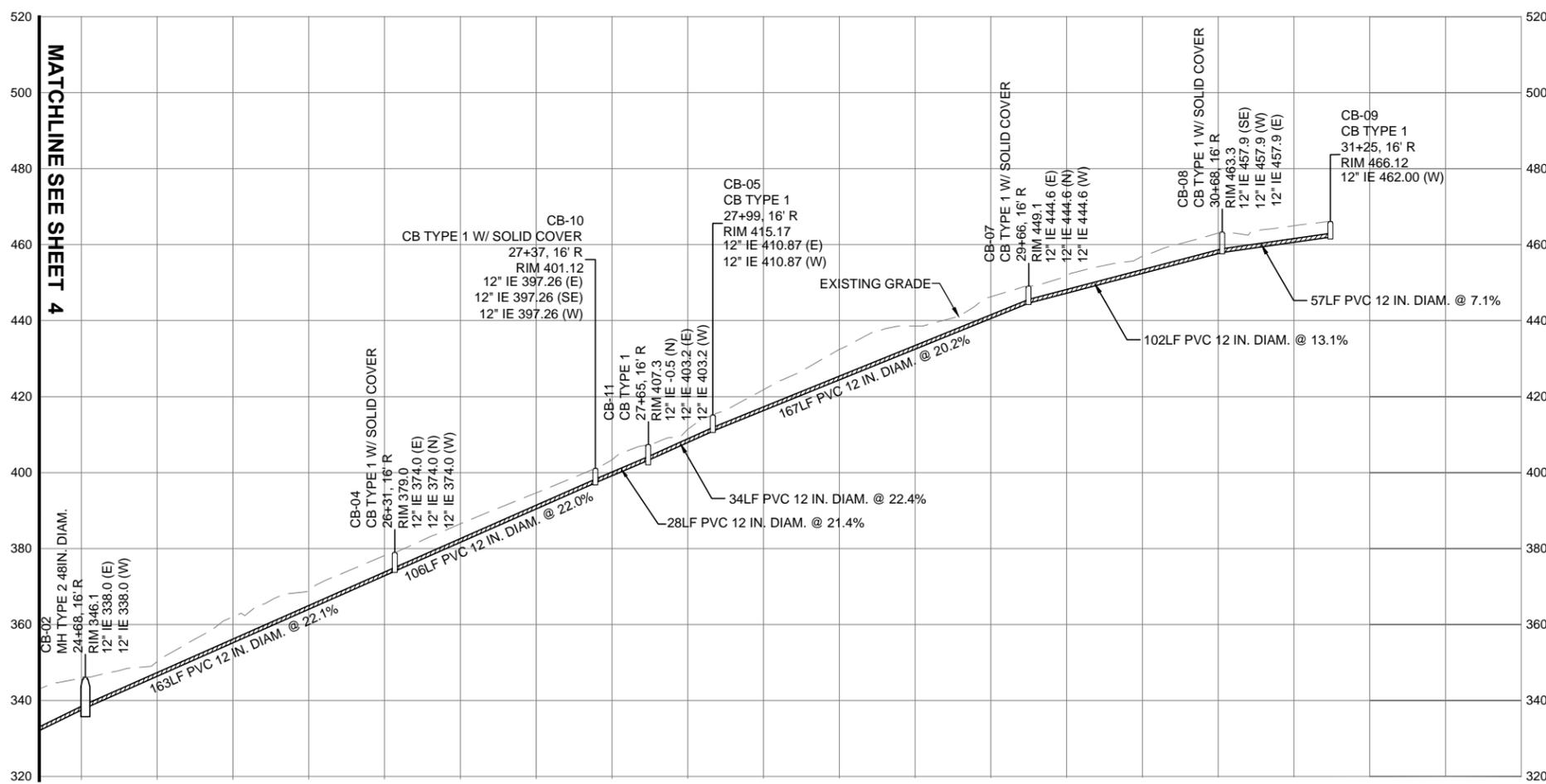
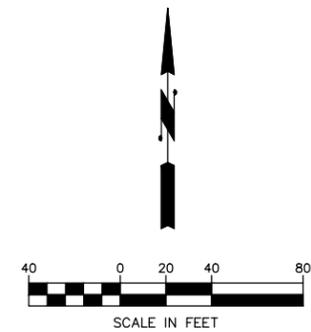
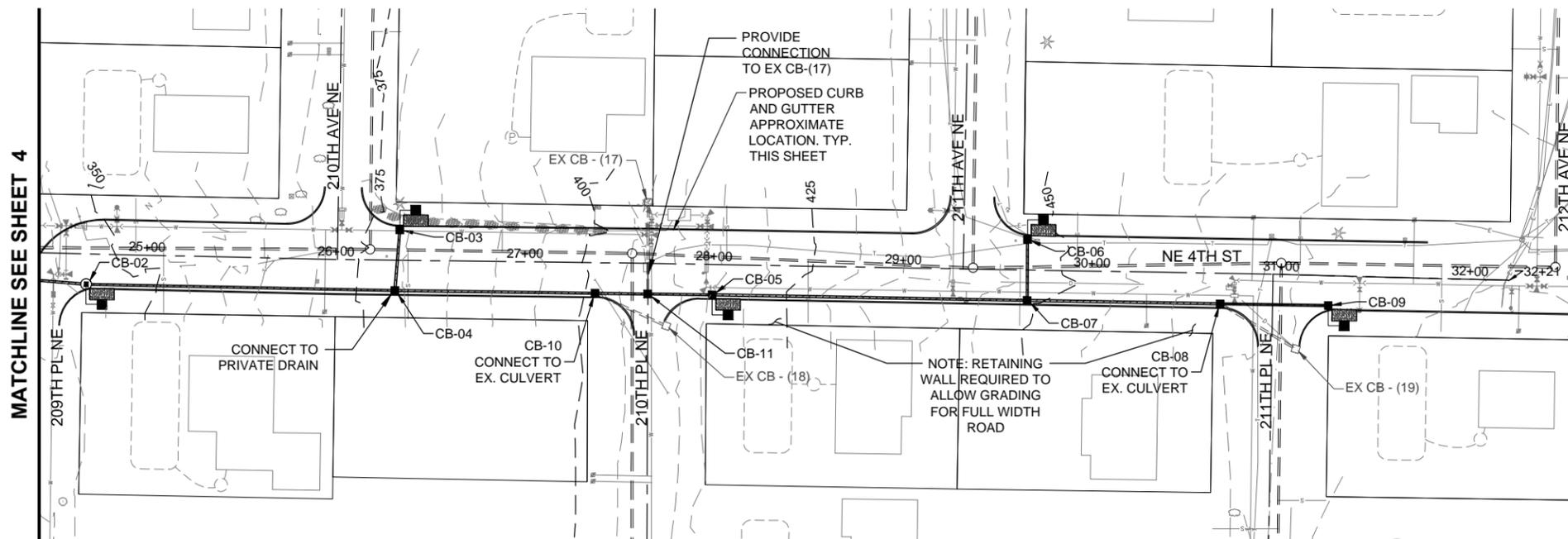
DESIGNED BY: RDP  
 DRAWN BY: MLP  
 CHECKED BY: LCR  
 DATE: DEC 2012

**TAMARACK**  
**STORMWATER IMPROVEMENTS**  
**209TH AVE NE**

**10-110024**  
 OCI PROJECT NO.  
 5 OF 7  
 SHEET OF

LEGEND:

- 25-FT CONTOUR (GIS)
- 5-FT CONTOUR (GIS)
- RIGHT-OF-WAY (GIS)
- ROAD CENTERLINE
- EXISTING PAVEMENT
- EXISTING SIDEWALK
- EXISTING SEWER MH
- EXISTING SEWER PIPE
- EXISTING STORM MH
- EXISTING STORM CB
- EXISTING STORM DRAIN PIPE
- PROPOSED MH TYPE 2
- PROPOSED CB TYPE 1
- PROPOSED WATER QUALITY TREATMENT 4-FT X 4-FT (SEE DETAILS SHEET 7)
- PROPOSED CONCRETE FLUME (SEE DETAILS SHEET 7)
- PROPOSED STORM DRAIN PIPE
- PROPOSED DITCH
- PROPOSED CURB AND GUTTER



SCALE BARS INDICATE SCALE OF FULL-SIZE (22 X 34 INCH) DRAWINGS. FOR REDUCED SIZE DRAWINGS, ADJUST SCALE ACCORDINGLY.

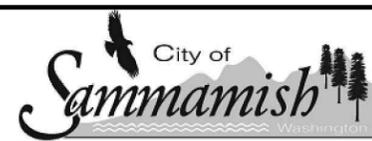
PRELIMINARY PLANS – NOT FOR CONSTRUCTION

FILE NAME: O:\Projects\10-110024\_Sammamish SW(CAD)\Tamarack\Tamarack.dwg  
 USER: Marie  
 PLOTTING DATE: 4/22/2013

Call 2 Working Days Before You Dig  
**1-800-424-5555**  
 Utilizes Underground Location Center (ID,MT,ND,OR,WA)



NO.	DATE	BY	CKD.	REVISION

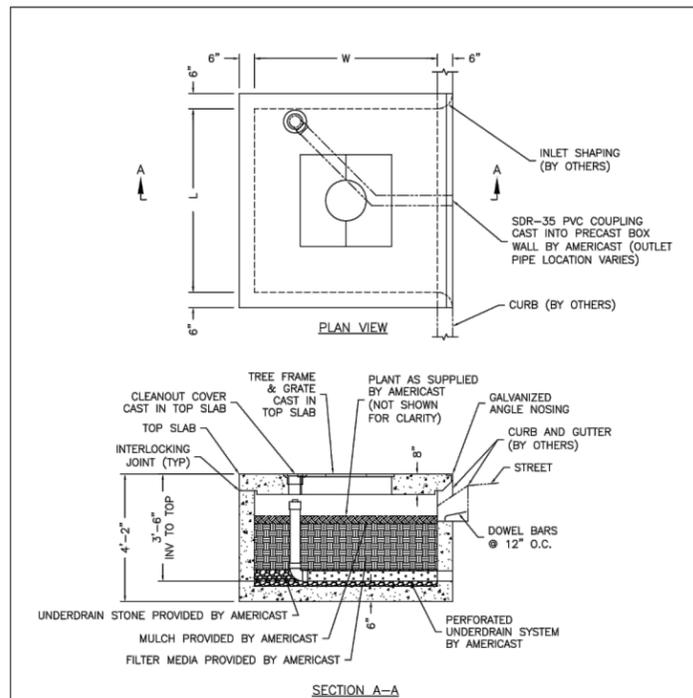


ENGINEER: Osborn Consulting Inc.  
 1800 - 112th Avenue NE, Suite 220E  
 Bellevue, Washington 98004  
 TEL: (425) 451 - 4009  
 FAX: (425) 451 - 4901

DESIGNED BY: RDP  
 DRAWN BY: MLP  
 CHECKED BY: LCR  
 DATE: DEC 2012

**TAMARACK**  
**STORMWATER IMPROVEMENTS**  
**NE 4TH ST**

**10-110024**  
 OCI PROJECT NO.  
 6 SHEET OF 7

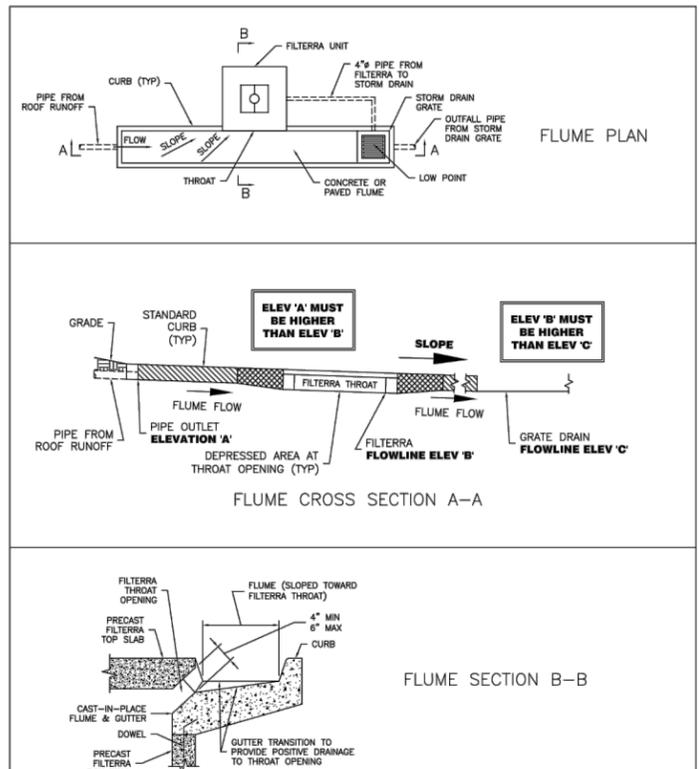


DESIGNATION	L	W	TREE GRATE QTY & SIZE	OUTLET PIPE
4 x 4	4'-0"	4'-0"	(1) 3x3	4" SDR-35 PVC
6 x 6	6'-0"	6'-0"	(1) 3x3	4" SDR-35 PVC

MODIFICATIONS OF DRAWINGS ARE ONLY PERMITTED BY WRITTEN AUTHORIZATION FROM FILTERRA. DRAWING AVAILABLE IN TIF FILE FORMAT.



DATE: 12-09-09 DWG: WWA FTST-3  
**PRECAST FILTERRA® UNIT  
 STANDARD CONFIGURATION  
 WESTERN WASHINGTON**  
 filterra®  
 US PAT 6,277,274 AND 6,569,321



MODIFICATIONS OF DRAWINGS ARE ONLY PERMITTED BY WRITTEN AUTHORIZATION FROM FILTERRA.



DATE: 03-17-05 DWG: FTRDF-1  
**FILTERRA® EXAMPLE SCENARIOS  
 ROOF DRAIN FLUME  
 APPLICATION**  
 filterra®  
 US PAT 6,277,274 AND 6,569,321

FILE NAME: C:\Projects\10-110024\_Sammamish SW\DWG\Tamarack\Tamarack.dwg  
 USER: Marie  
 PLOTTING DATE: 4/22/2013

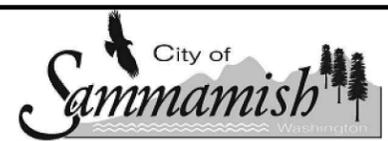
Call 2 Working Days Before You Dig  
**1-800-424-5555**  
 Utilizes Underground Location Center  
 (ID,MT,ND,OR,WA)



PRELIMINARY PLANS – NOT FOR CONSTRUCTION

SCALE BARS INDICATE SCALE OF FULL-SIZE (22 X 34 INCH) DRAWINGS. FOR REDUCED SIZE DRAWINGS ADJUST SCALE ACCORDINGLY.

NO.	DATE	BY	CKD.	REVISION



**OSBORN  
 CONSULTING  
 INCORPORATED**

ENGINEER: Osborn Consulting Inc.  
 1800 - 112th Avenue NE, Suite 220E  
 Bellevue, Washington 98004  
 TEL: (425) 451 - 4009  
 FAX: (425) 451 - 4901

DESIGNED BY: RDP  
 DRAWN BY: MLP  
 CHECKED BY: LCR  
 DATE: DEC 2012

**TAMARACK  
 STORMWATER IMPROVEMENTS  
 DETAILS**

**10-110024**  
 OCI PROJECT NO.  
 7 / 7  
 SHEET OF

**ATTACHMENT B**

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**TAMARACK NEIGHBORHOOD DRAINAGE  
PROJECT COST ESTIMATE**

**Opinion (Estimate) of Probable Cost**

		<b>Project No.</b> Sammamish Neighborhood Projects	<b>Date</b> December 21, 2012			
<b>Project Name</b>		Tamarack Neighborhood				
<b>Location</b>		NE 4th St and 209th Ave NE, Sammamish, WA				
<b>Owner</b>		City of Sammamish				
<b>Estimated By:</b>		Joe Wright	<b>Checked By:</b>		Laura Ruppert	<b>Approved By:</b> L. Ruppert
<b>Date:</b>		December 20, 2012	<b>Date:</b>		12/21/12	<b>Date:</b> April 2013
ITEM NO.	SPEC SECTION	DESCRIPTION	QTY	UNIT	UNIT PRICE	TOTAL COST
		Mobilization	1	LS	8%	\$38,507.14
		Clearing and grubbing	0.1	ACRE	\$8,500.00	\$850.00
		Removing Asphalt Conc. Pavement	1,610	SY	\$6.00	\$9,658.00
		Ditch Excavation Incl. Haul	30	CY	\$25.00	\$740.74
		Cement Conc. Curb and Gutter	1,590	LF	\$30.00	\$47,700.00
		Schedule A Storm Sewer Pipe 12 in. Diam*	1,056	LF	\$40.00	\$42,240.00
		Schedule A Storm Sewer Pipe 18 in. Diam*	1,813	LF	\$60.00	\$108,780.00
		Schedule A Storm Sewer Pipe 24 in. Diam*	134	LF	\$70.00	\$9,380.00
		Pipe Anchor	6	Each	\$3,000.00	\$9,000.00
		Catch Basin Type 1	11	Each	\$1,000.00	\$11,000.00
		Catch Basin Type 2 48 In. Diam.	3	Each	\$2,200.00	\$6,600.00
		Connect to existing catch basin or manhole	24	Each	\$620.00	\$14,880.00
		Structure Excavation Class B Incl. Haul	1,690	CY	\$9.00	\$15,210.00
		Crushed surfacing base course	268	CY	\$30.00	\$8,048.33
		HMA For Pavement Repair Cl. 3/4 In. PG	642	TON	\$150.00	\$96,244.65
		Temporary Erosion and Sediment Control	1	EST	\$6,000.00	\$6,000.00
		Roadside Restoration	1	EST	\$6,000.00	\$6,000.00
		Project Temporary Traffic Control	1	EST	\$6,000.00	\$6,000.00
		Plugging Existing Pipe	1	Each	\$500.00	\$500.00
		Shoring or Extra Excavation Class B	15,015	SF	\$0.50	\$7,507.50
		Water Quality Structure (Filterra 4'x4')	5	Each	\$15,000.00	\$75,000.00
		<b>Subtotal Project Cost</b>				<b>\$519,846.37</b>
		<b>Contingency</b>			<b>30%</b>	<b>\$155,953.91</b>
		<b>Tax</b>			<b>10%</b>	<b>\$64,201.03</b>
		<b>Easement Acquisition</b>				<b>\$0.00</b>
		<b>Engineering Design</b>			<b>20%</b>	<b>\$103,969.27</b>
		<b>Permitting</b>			<b>5%</b>	<b>\$25,992.32</b>
		<b>Construction Management</b>			<b>20%</b>	<b>\$103,969.27</b>
		<b>TOTAL PROJECT COST</b>				<b>\$973,932.16</b>
		<b>TOTAL PROJECT COST (Rounded)</b>				<b>\$974,000.00</b>

**Notes:**

\*Includes installation, materials, pipe zone bedding, trench backfill and CDF and SD testing

## ATTACHMENT C

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### HYDROLOGIC AND HYDRAULIC ANALYSIS

The Hydrologic and Hydraulic Analysis was provided by Windward Environmental. This attachment contains the following:

- Flows generated from MGSFlood
- Existing pipe sizing and capacity calculations
- Proposed pipe sizing and capacity calculations

<b>100-Year Storm Event Flows</b>				
	<b>Tlingit Subdivision</b>	<b>Private Road</b>	<b>209th Ave NE</b>	<b>NE 4<sup>th</sup> Street</b>
<b>100-Year Flows (cfs)</b>	19.223	10.735	9.358	6.189

## Existing Pipes - Assuming Reusing CMP

Pipe	Diameter	Area(SF)	Cumulative Flow out (cfs)				n	0.022
			Tam7 Q	Tam5 Q	Tam 4	Tam 3		
			6.189	9.358	10.735	19.223		
12"	1	0.785	8.7%	19.8%	26.0%	83.5%		
18"	1.5	1.76625	1.00%	2.28%	3.0%	9.6%		
24"	2	3.14	0.21%	0.49%	0.6%	2.1%		
36"	3	7.065	0.02%	0.06%	0.1%	0.2%		

NE 4th street - 22.0%

Private stairs - 28.0%

Private road - 0.5%

NE 5th PL - 15.0%

206th ave NE - 5.0%

205th ave NE - 1.0%

- Pipe diameter meets slopes of area
- Slope will have to be adjusted or pipe size increased

### Mannings Equation (for partially full pipe)

$$q = Av = A * k_n / n * R^{2/3} * S^{1/2}$$

q	CFS	flow	R	Ft	hydraulic radius = A/P
A	SF	cross sectional area	P	Ft	wetted perimeter
v	ft/sec	average velocity	n		mannings roughness
k <sub>n</sub>	1.486	for English units	S <sup>1/2</sup>		sqrt of slope

## Proposed Pipes - Assuming Using HDPE

Pipe	Diameter	Area(SF)	Cumulative Flow out (cfs)				n	0.013
			Tam7 Q	Tam5 Q	Tam 4	Tam 3		
			6.189	9.358	10.735	19.223		
12"	1	0.785	3.0%	6.9%	9.1%	29.1%		
18"	1.5	1.76625	0.35%	0.79%	1.0%	3.4%		
24"	2	3.14	0.07%	0.17%	0.2%	0.7%		
36"	3	7.065	0.01%	0.02%	0.0%	0.1%		

NE 4th street - 22.0%

Private stairs - 28.0%

Private road - 0.5%

NE 5th PL - 15.0%

206th ave NE - 5.0%

205th ave NE - 1.0%

- Pipe diameter meets slopes of area
- Slope will have to be adjusted or pipe size increased

### Mannings Equation (for partially full pipe)

$$q = Av = A * k_n / n * R^{2/3} * S^{1/2}$$

q	CFS	flow	R	Ft	hydraulic radius = A/P
A	SF	cross sectional area	P	Ft	wetted perimeter
v	ft/sec	average velocity	n		mannings roughness
k <sub>n</sub>	1.486	for English units	S <sup>1/2</sup>		sqrt of slope

**ATTACHMENT 8**

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**FILTERRA® BIORETENTION SYSTEMS**

# CALCULATIONS COVER PAGE

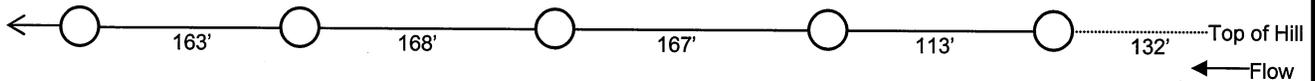


REVISION 0 (11/9/2012)

<b>PROJECT:</b> Sammamish SW - Tamarack	<b>OCI JOB NO.:</b> 10-110024	<b>PLAN NO.:</b>	<b>PAGE 1 OF 5</b> Total Pages includes Attachments.	
<b>CLIENT:</b> City of Sammamish	<b>DEPARTMENT/DISCIPLINE:</b>	<b>CALCULATION NO.</b>		
<b>SUBJECT/TITLE:</b> WATER QUALITY STRUCTURE SPACING				
<b>CALCULATION REV. NO.</b>	<b>ORIGINATOR</b>	<b>DISCIPLINE REVIEWER</b>	<b>TECHNICAL PEER REVIEWER (IF REQUIRED)</b>	<b>CONFIRMATION REQUIRED (Y/N) IF YES, INCLUDED ATTACHMENT II</b>
0	M. Phelan	L. Ruppert	N/A	N
1				
2				
3				
<b>CALCULATIONS OBJECTIVE</b>				
Determine size and spacing needed for Filterra boxes, based on PGIS.				
<b>CALCULATION METHODOLOGY/ LIST OF ASSUMPTIONS</b>				
<p>Assumptions:</p> <ol style="list-style-type: none"> <li>1. 20-foot pavement width + 16-foot parking = 36-foot PGIS (See Page 3)</li> <li>2. Available Filterra sizes are 4x4 and 4x6 (See Pages 4 and 5)</li> <li>3. Sized for Enhanced Treatment to account for sensitive Lake WQ treatment (See Page 4)</li> <li>4. Road is crowned, and half the road width contributes to each Filterra system (See Page 3)</li> </ol> <p>Following is the process used to size the boxes:</p> <ol style="list-style-type: none"> <li>1. Determine manhole spacing.</li> <li>2. Determine maximum spacing for Filterra systems</li> <li>3. Determine the Filterra layout</li> <li>4. Determine the cost</li> </ol>				
<b>REFERENCES / INPUTS</b>				
<p><b>Roadway Width</b></p> <p>Roadway width from City of Sammamish Department of Public Works Standards, FIG01-05. Assumed parking but no sidewalks (See Page 3).</p>				

### Manhole Spacing

From AutoCAD drawing. Total Distance is 774 feet.



### Filtterra Spacing

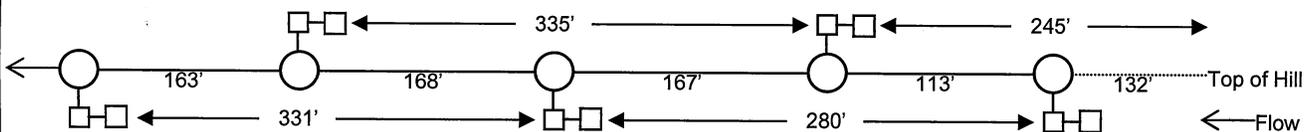
Maximum Spacing from Engineering Design Assistance Kit from Filtterra Bioretention Systems, Table 1: WWHM Sizing for Basic Treatment – TSS, Oil/Grease and Phosphorous Treatment (See Page 4). Multiply spacing by two to account for half the road width.

### Cost

Filtterra Box costs from Filtterra Bioretention Systems, Filtterra Price List – Standard Unity, Pacific Northwest Region (See Page 5).

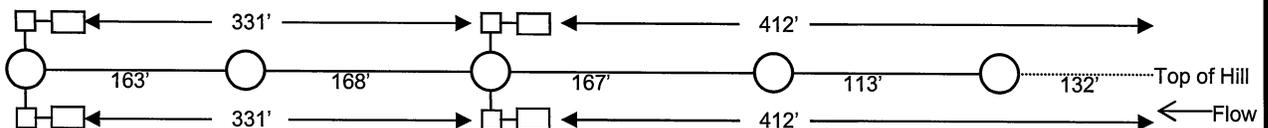
## CONCLUSIONS

For 18-foot pavement width, the spacing for a 4x4 Enhanced Treatment Filtterra Box is 339 feet. The below diagram shows the spacing with respect to the proposed manholes.



The total cost for five boxes, plus 50% for offloading, installation, and sales tax (See Page 5) is \$62,250.

For 18-foot pavement width, the spacing for a 4x6 Enhanced Treatment Filtterra Box is 509 feet. The below diagram shows the spacing with respect to the proposed manholes.



The total cost for four boxes, plus 50% for offloading, installation, and sales tax (See Page 5) is \$56,100.

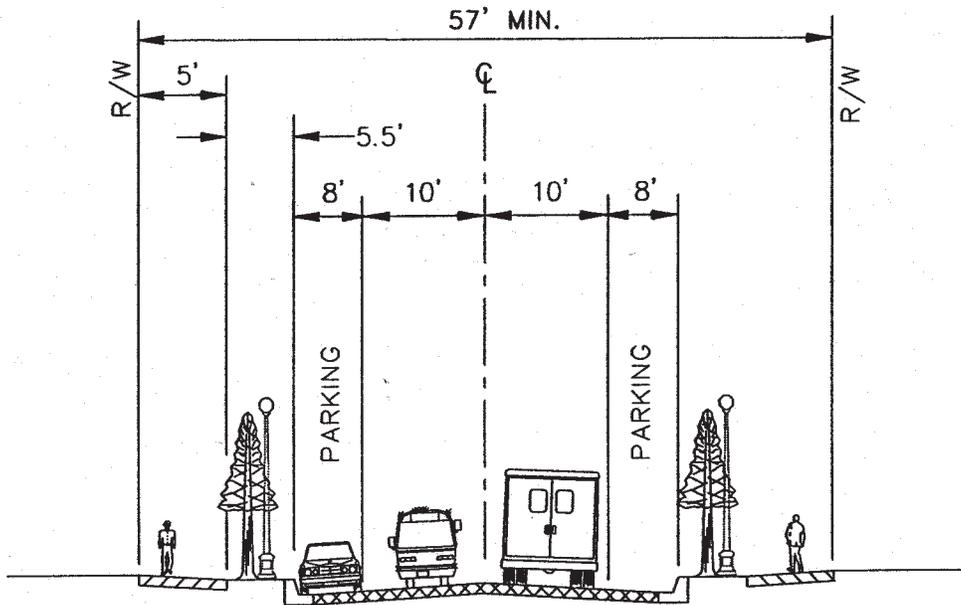
## RECOMMENDATION

OCI recommends using the **4x4 Filtterra Box** because it is unlikely that the runoff will sheet flow over the intersections, which it would need to do with the 4x6 boxes.

**THE CALCULATIONS IS COMPLETED AND READY FOR DISCIPLINE REVIEW**

Originator *Marie Phelan* 11/9/12

Signature/Date



**ROADWAY SECTION  
LOCAL ROAD  
DETAIL**

N.T.S.



EXPIRES: 4/30/01

**NOTES:**

1. ADD 5' OF PAVEMENT WIDTH EACH SIDE AND 10' OF RIGHT-OF-WAY WIDTH WHEN BIKE LANES ARE REQUIRED.
2. ON-STREET PARKING MAY BE REDUCED WITH CITY ENGINEER'S APPROVAL FOR CUL-DE-SAC STREETS.

CITY OF SAMMAMISH DEPARTMENT OF PUBLIC WORKS			
ROADWAY SECTION LOCAL ROAD			
APPROVED BY CITY ENGINEER		DATE	
DWN JM	CKD SPS	DATE MARCH-15-2000	FILE FIG01-05

REV



**Table 2: WWHM Sizing for Enhanced Treatment - Dissolved Metals**  
**Western Washington Region ONLY - v01a**

Available Filterra® Box Sizes (feet)	Approximate Contributing Drainage Area (acres)	Maximum Spacing (18-ft pore width)
4 x 4	0.140	339 LF
4 x 6 or 6 x 4	0.210	509 LF
4 x 8 or 8 x 4	0.275	
6 x 6	0.310	
6 x 8 or 8 x 6	0.415	
6 x 10 or 10 x 6	0.520	
6 x 12 or 12 x 6	0.630	

Notes:

1. Sizing table intended for planning level use. The design engineer must use the latest version WWHM to calculate the appropriately sized facility.
2. Sizing table meets WA DOE 2005 Stormwater Manual's 91% annual stormwater volume filtered.
3. Sizing table based on WWHM3 parking/flat and the SeaTac rain gauge with a precipitation factor of 1.0. Other precipitation factors, geographic locations and site conditions will affect Filterra sizing.
4. Sand Filter (Filterra) parameters:
  - Filter material depth = 1.8 feet
  - Effective ponding depth = 0.75 feet
  - Zero slope(s) on the filter box
  - Riser height = 0.7 feet
  - Riser diameter = 100 inches
  - Filter Hydraulic Conductivity = 24.82 inches per hour
5. All boxes are a standard 3.5 feet depth (INV to TC).
6. A standard SDR-35 PVC pipe coupling is cast into the wall for easy connection to discharge drain.
7. Dimensions shown are internal. Please add 1' to each external (using 6" walls).
8. Valid for Enhanced Treatment regiments (Dissolved Zinc and Copper).
9. For sizing in other areas of Washington State please contact Filterra.



## Filterra Price List – Standard Units

### Pacific Northwest Region

Effective May 1, 2011

<b>4 x 4 .....\$8,300</b>	<b>4 x 12 or 12 x 4.....\$14,200</b>
<b>4 x 6 or 6 x 4.....\$9,350</b>	<b>6 x 8 or 8 x 6.....\$13,800</b>
<b>4 x 8 or 8 x 4.....\$10,050</b>	<b>6 x 10 or 10 x 6.....\$17,500</b>
<b>3 x 8 or 8 x 3.....\$10,900</b>	<b>6 x 12 or 12 x 6.....\$20,450</b>
<b>6 x 6 Standard.....\$10,400</b>	<b>7 x 13 or 13 x 7.....\$23,400</b>

### Notes

1. Price includes: Concrete box & top, engineered media, suitable mulch, internal underdrain system & clean out, plant, standard tree grate and two maintenance visits in the first year of each Filterra unit (Maintenance agreement is between owner of Filterra® and Americast) . Delivery to site is included.
2. Modifications to standard products may incur an extra charge and prepayment prior to manufacture (note local tree grate standards).
3. Price does not include offloading, installation or sales taxes (where applicable).
4. The contractor is responsible for safe unloading, handling and installation and to determine if a crane is required. Refer to table of weights of components. Lifting Filterra® boxes **always** requires a spreader bar (not included).
5. The plant/tree and mulch (activation) will be installed by Americast only after the Filterra® units are installed and the site is fully stabilized (full landscaping, grass cover, final paving and street sweeping completed).
6. The total unit price is due to Americast upon shipment or at 3 months after release for production, whichever is sooner. Activation, maintenance or any other services performed by Americast do not constitute a reason for delay of payment.
7. Filterra® is protected under U.S. Patents # 6,277,274, 6,569,321, 7,425,261, 7,625,485, 7,833,412 and D596,697.
8. This Price List supersedes all other Price Lists and is subject to change without notice.